

# Getting Familiar with the BCOA- ME Design Guide

Bonded Concrete Overlay of Asphalt Pavements  
Mechanistic-Empirical Design Guide (BCOA – ME)



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# **EFFECT OF FIBER ON JOINT PERFORMANCE**



# [ Benefits of structural fibers ]

Current: Increase MOR

## Advantage

- Increase fracture toughness
- Decrease crack/joint width
- Potential increase in load transfer

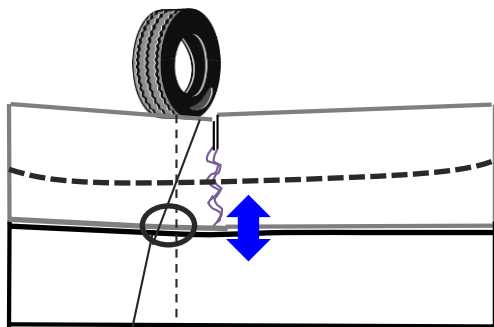


## Disadvantage

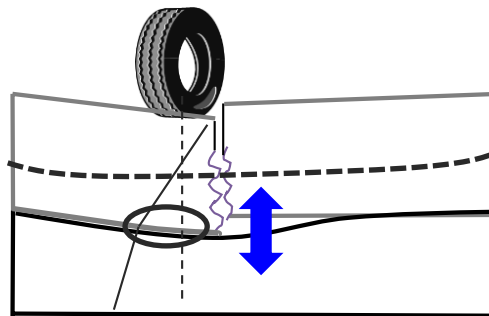
- Increase cost of PCC by approx. 20%

# Influence on load transfer

1. Reduce load related stress on the loaded slab;
2. Reduce debonding tensile “stresses between the layers.



High LTE

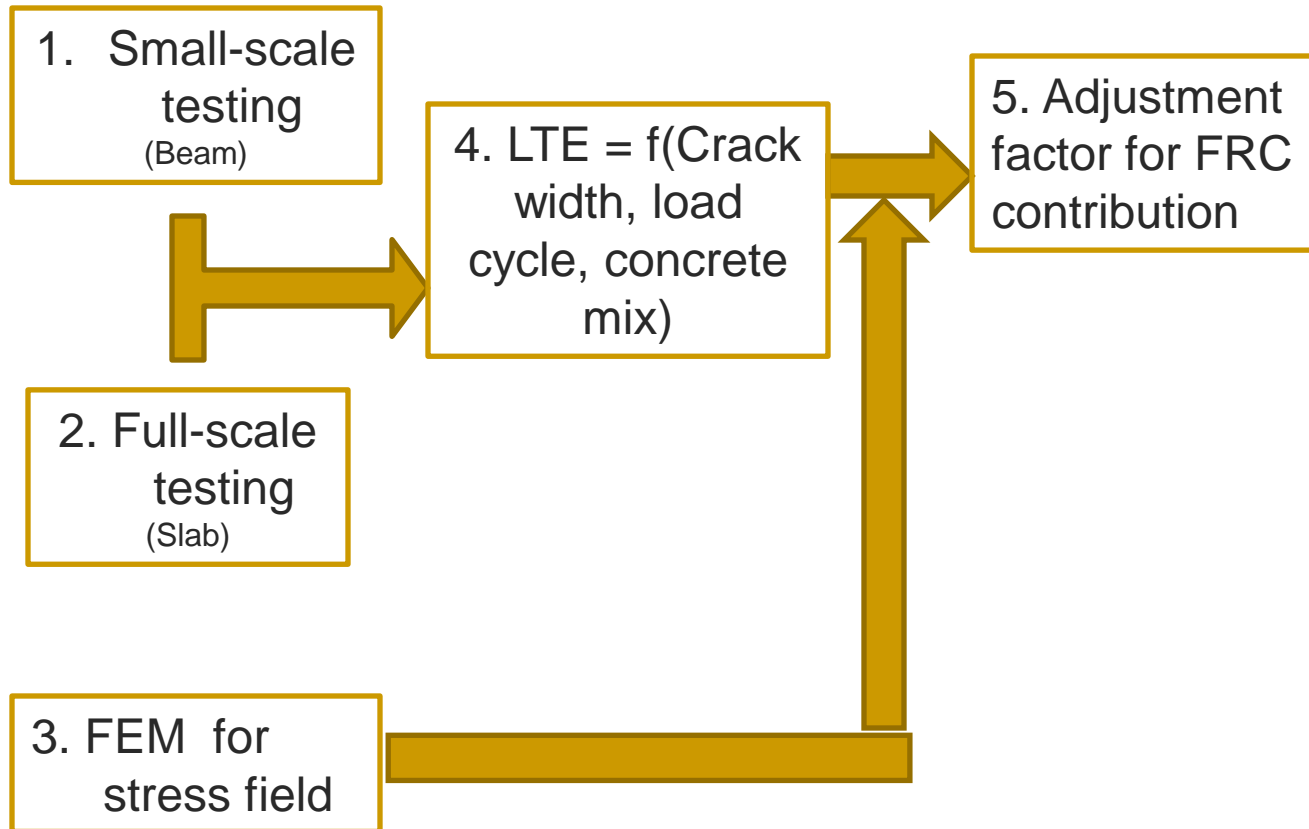


Low LTE

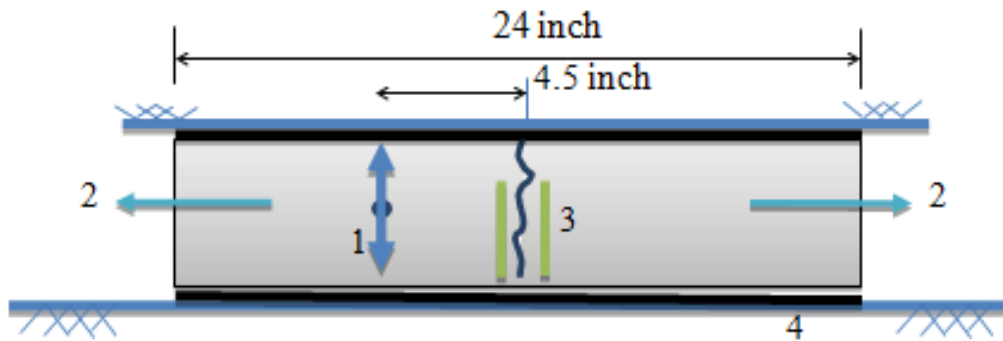
$$LTE = \frac{\Delta_U}{\Delta_L} \times 100 \text{ percent}$$

$\Delta_U$  and  $\Delta_L$  = def. at  
unloaded and loaded  
slabs.

# Strategy

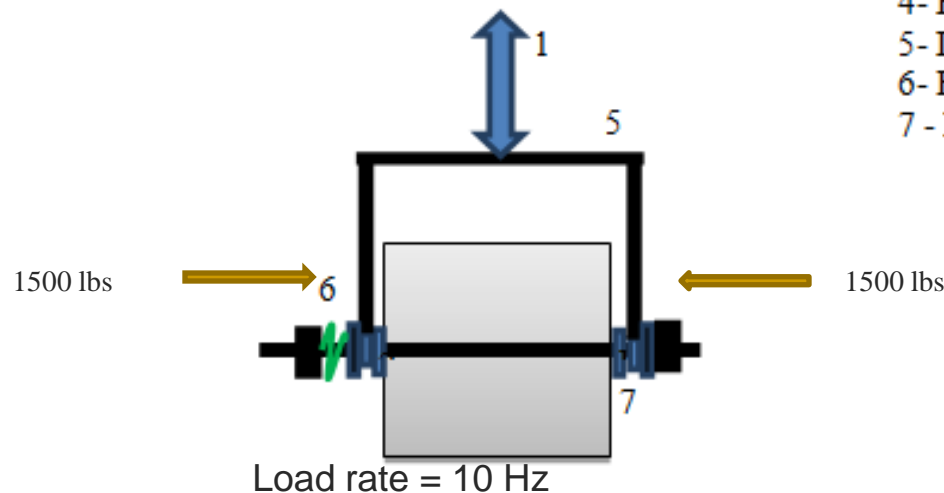


# 1. Development of small-scale test

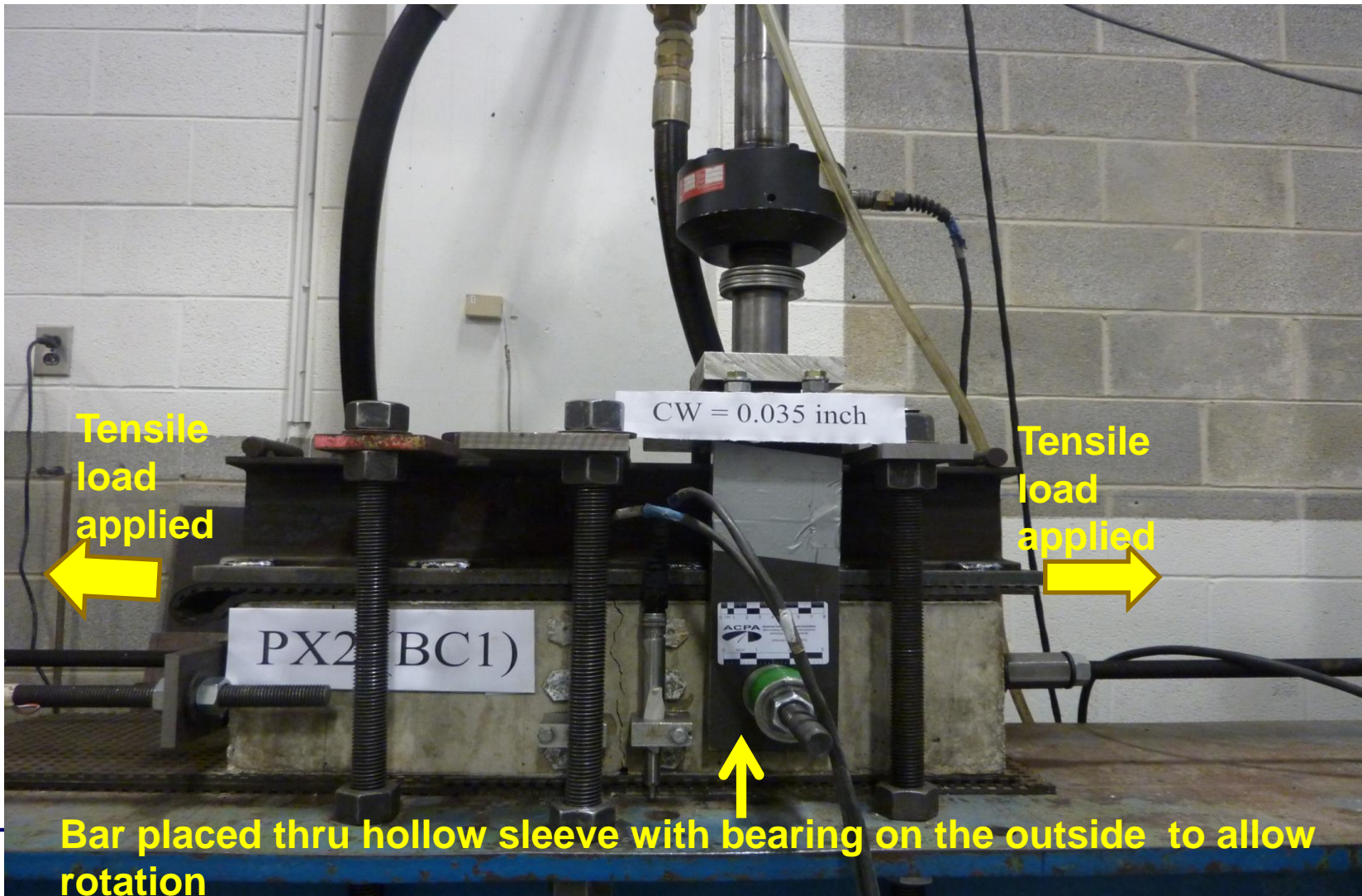


(a)

- 1-  $\pm 1050$  lbs load
- 2- Horizontal force
- 3- LVDTs
- 4- Fabcel, artificial foundation
- 5- Load plate
- 6- Horizontal load application spring
- 7 - Bearing

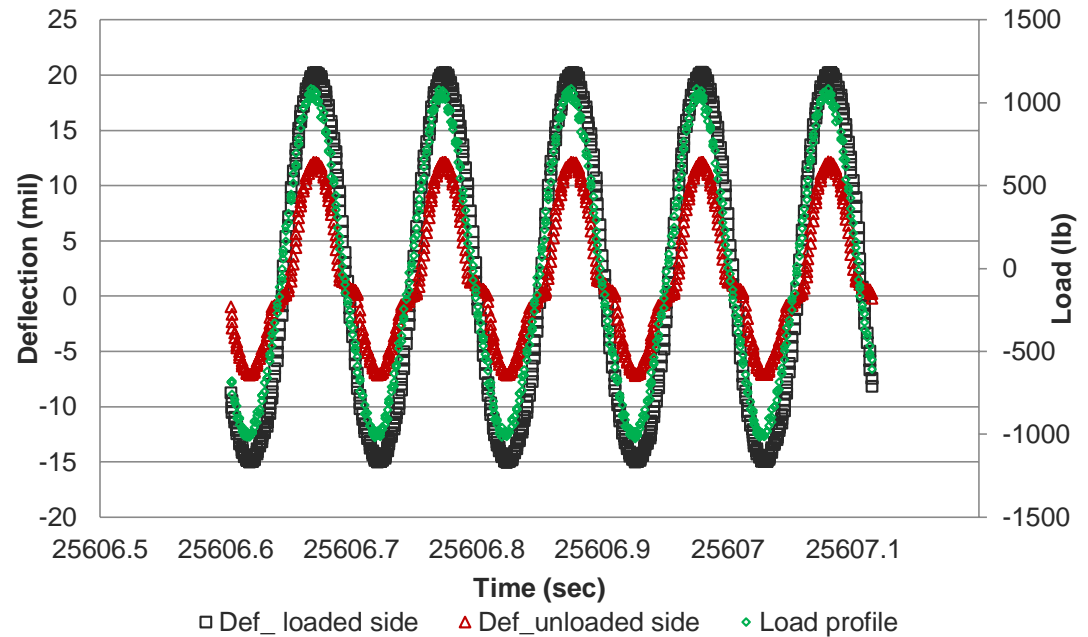


# 1. Small-scale LTE testing





# 1. Small-scale LTE testing





## 2. Large-scale testing



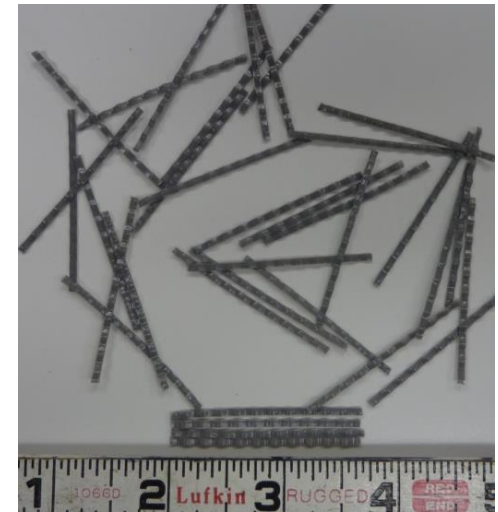
# Fibers Considered

Synthetic Fiber	Brand	Length (inch)	Shape	Cross section (inch x inch)	Specific gravity	Aspect ratio	Quantity (lb/cy)
Straight		1.57	Rectangular	0.05 x 0.004	0.92	90	
Crimped							



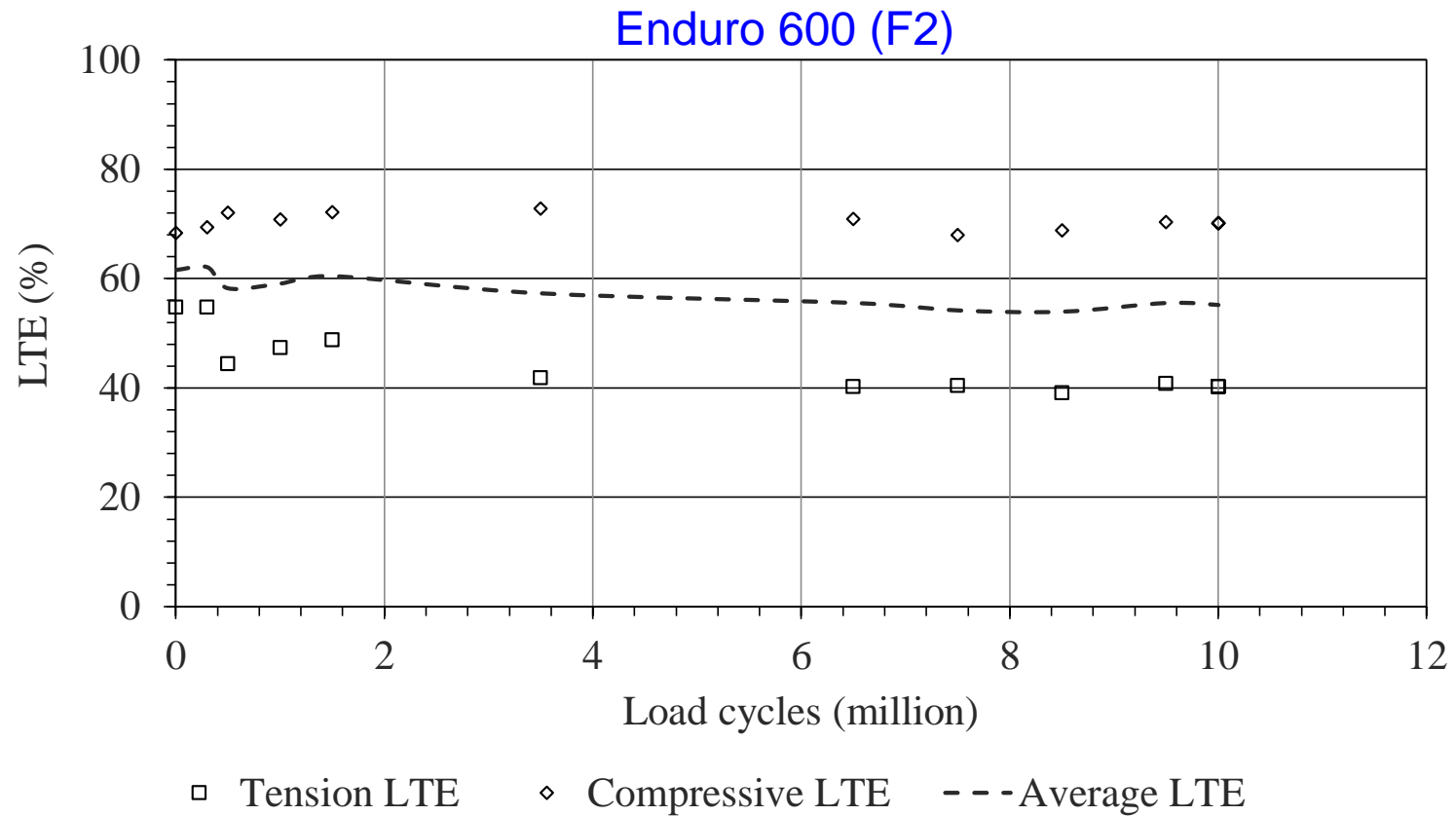
Strux 90/40 (F1)

Target residual strength = 20%

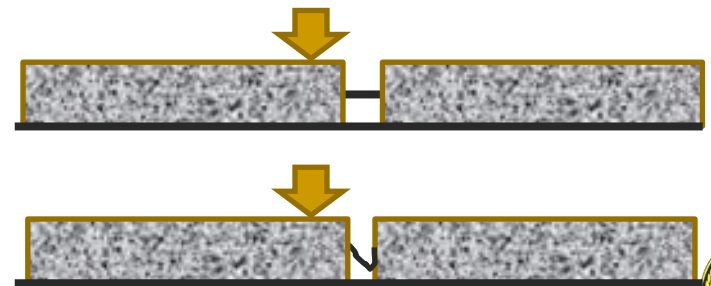
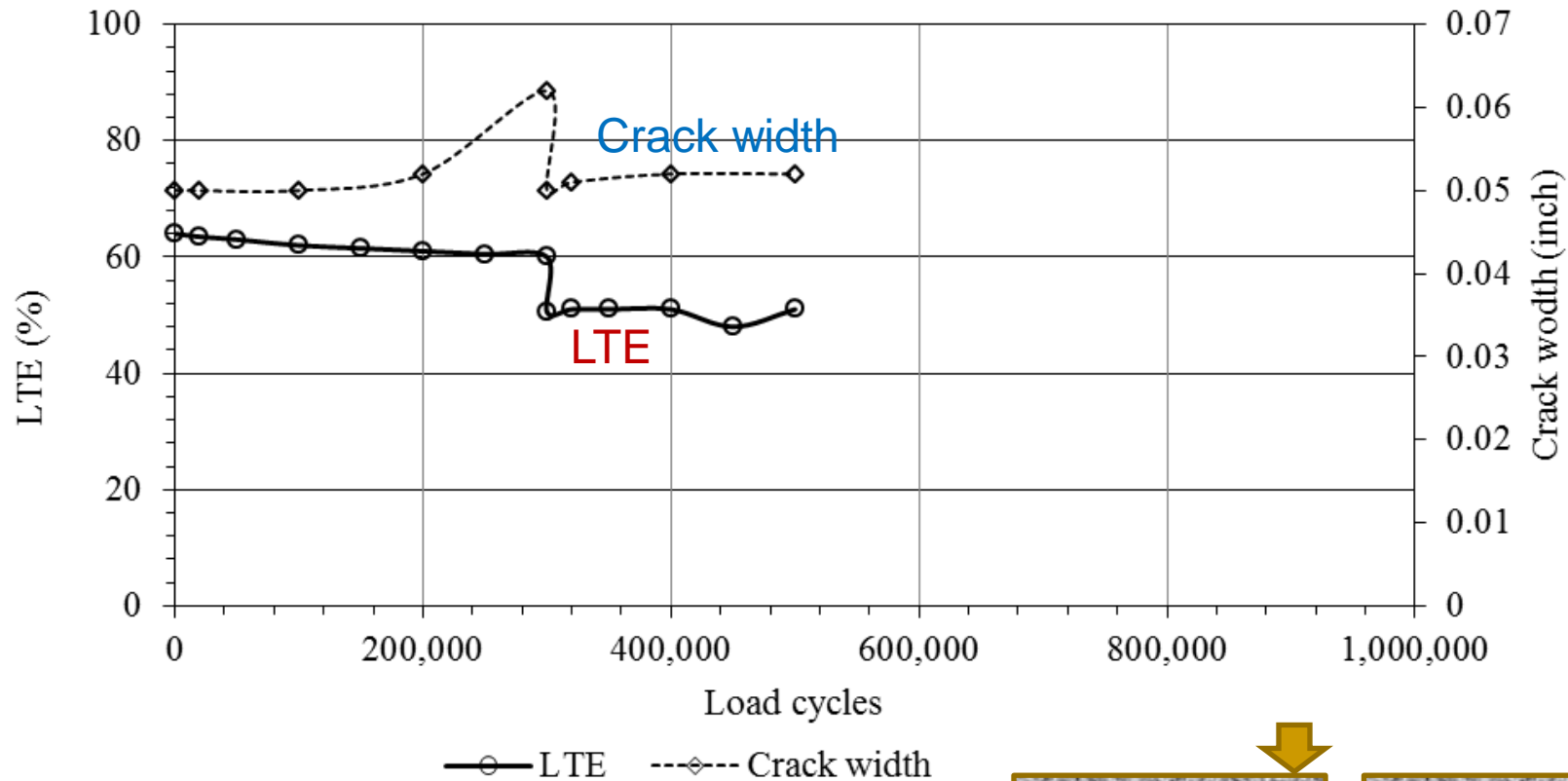


Enduro 600 (F2)

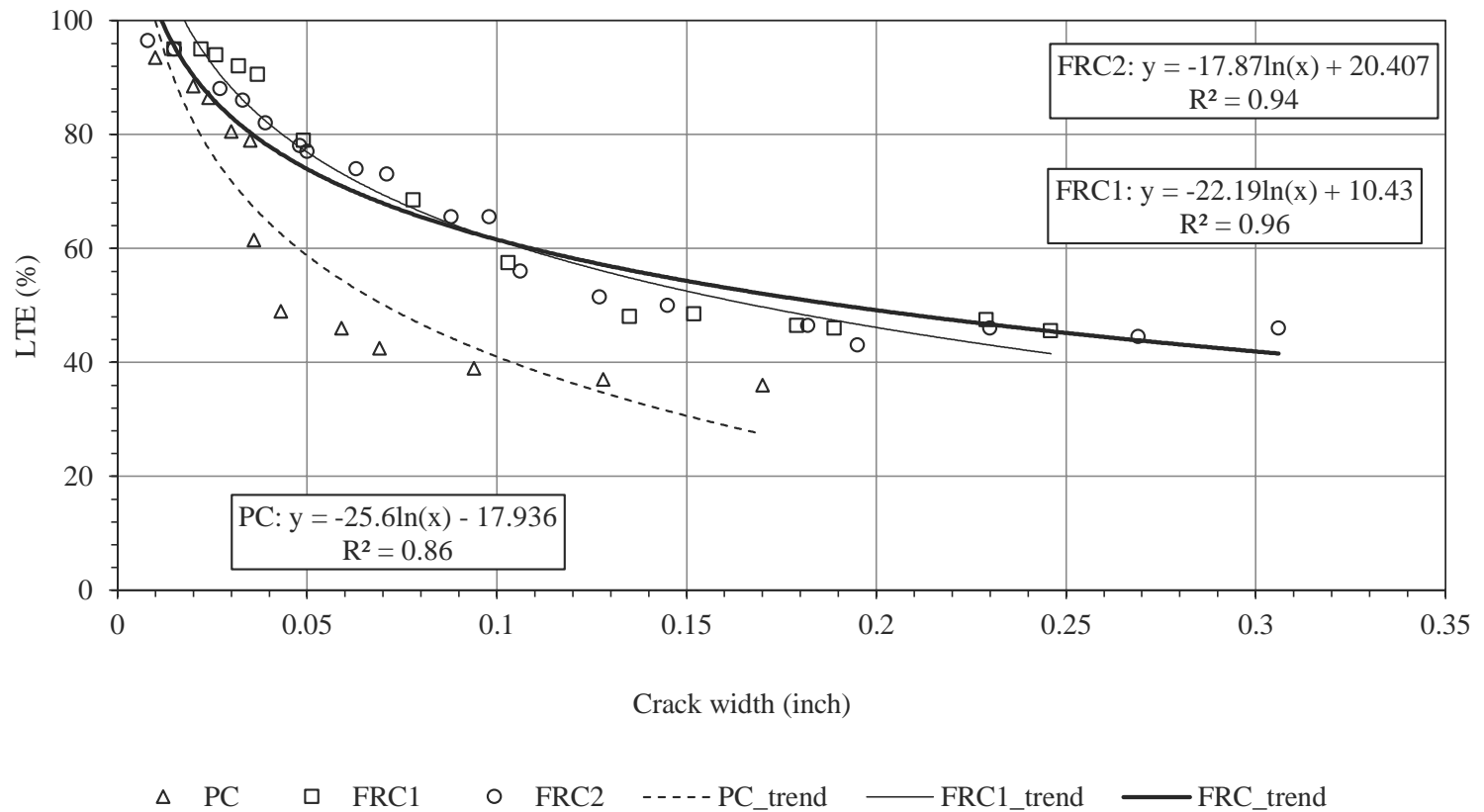
# No Fatigue of Fiber



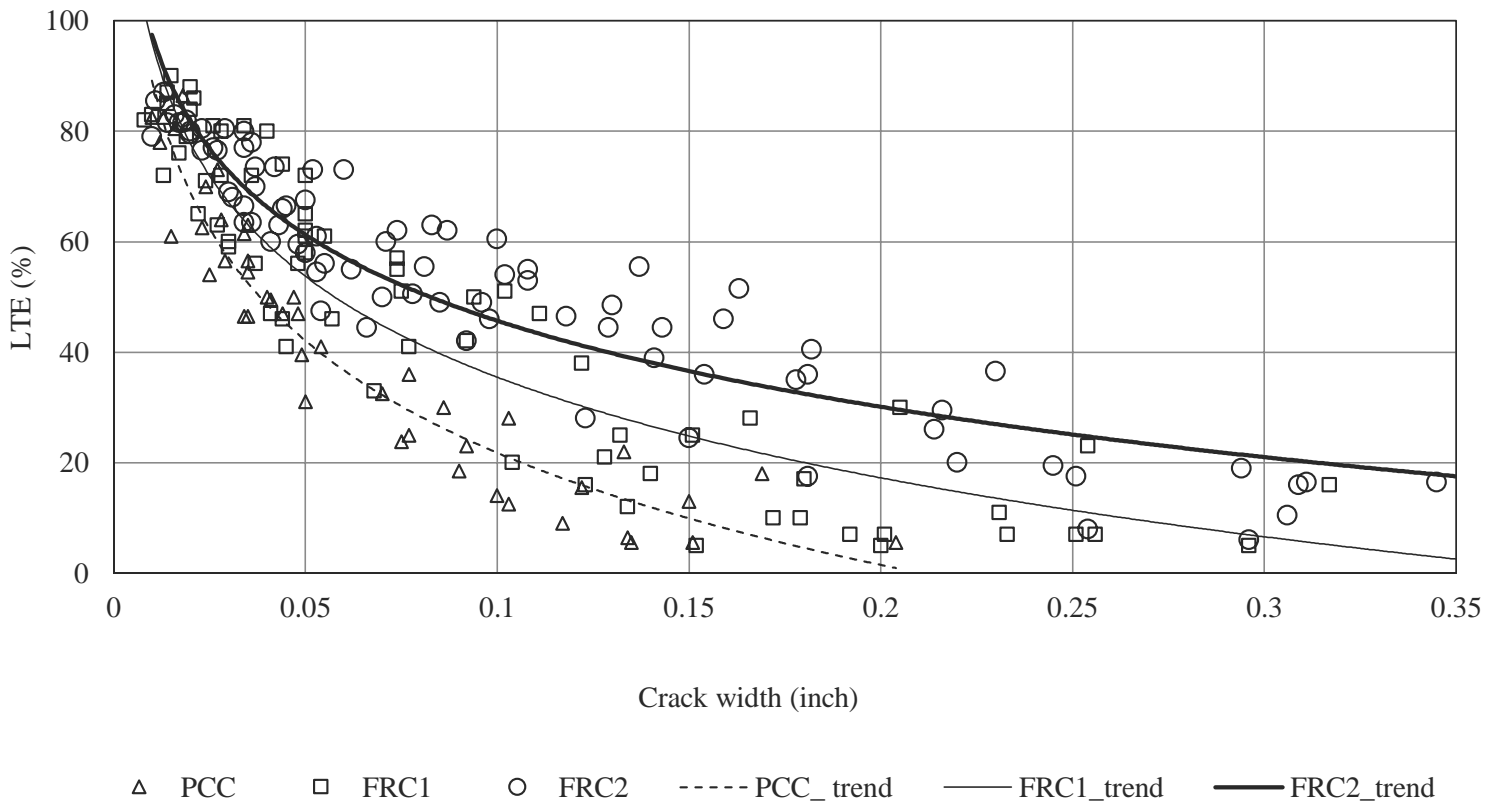
# Effects of reduction in crack width



# Slab performance



# Beam performance



# Comments

- Even though the shapes, sizes and aspect ratios of the two fibers were different, performances were similar.
- Residual strength fiber selection criteria could possibly indicate equivalent joint performance.
- Fibers increases LTE by 10%
- Fiber did not exhibit fatigue after 10 million load applications
- Effectiveness of fiber appears to decrease when crack width is less than max. crack opening experienced

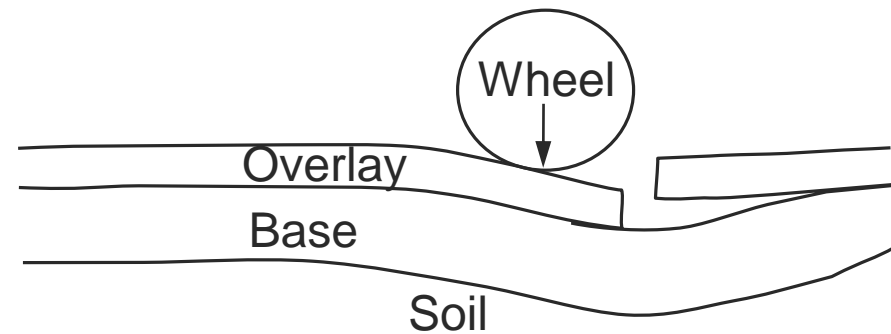


# DEBONDING MODEL



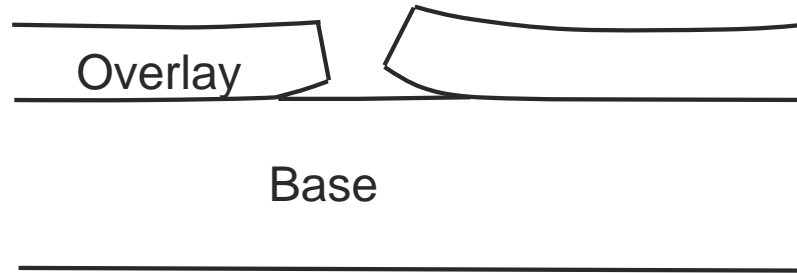
# Interface Debonding

**Debonding Force > Debonding Resistance**



## Tensile failure

1. Wheel load
2. Curling/warping

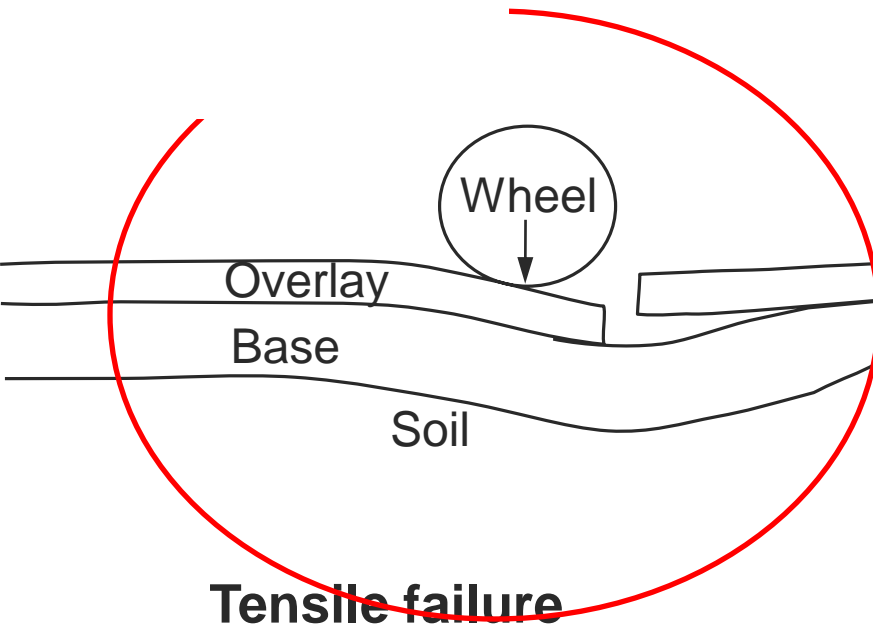


## Shear failure

1. Wheel brake
2. Differential length change

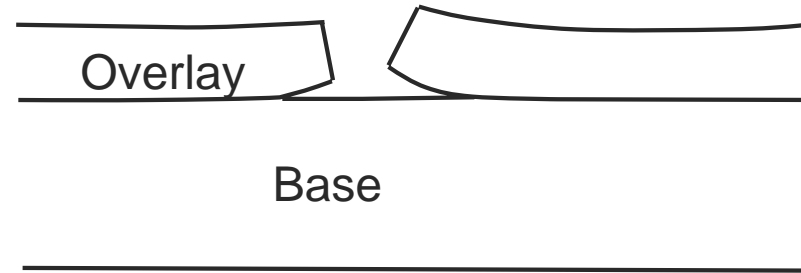
# Interface Debonding

**Debonding Force > Debonding Resistance**



## Tensile failure

1. Wheel load
2. Curling/warping



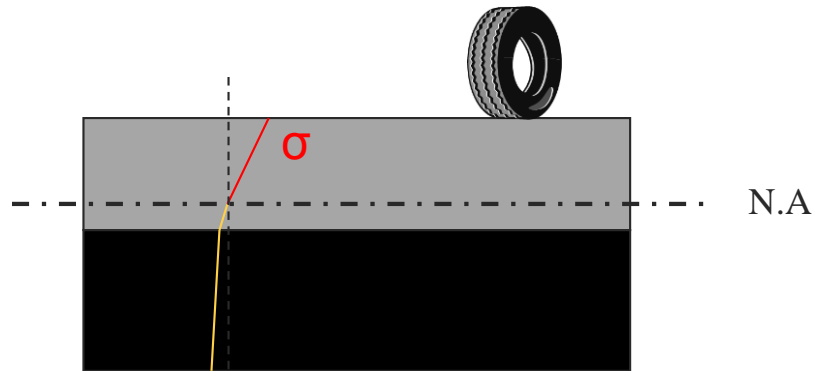
## Shear failure

1. Wheel brake
2. Differential length change

# [Interface bond]

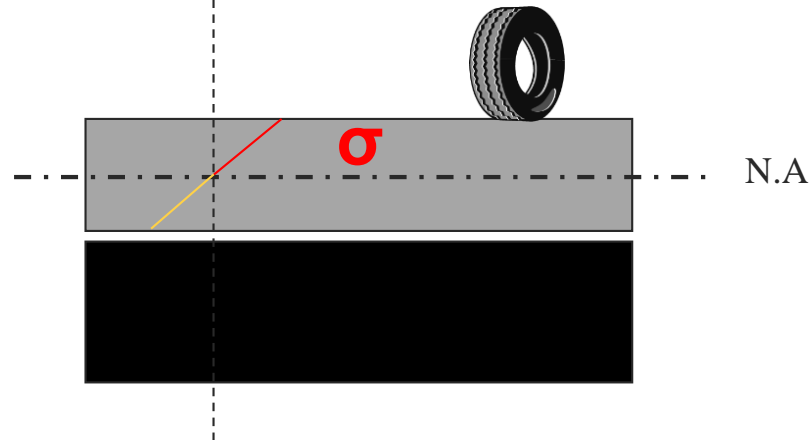
$$\sigma_{design} = f(\text{degree of bond}) \cdot \sigma_{bonded}$$

**Bonded**



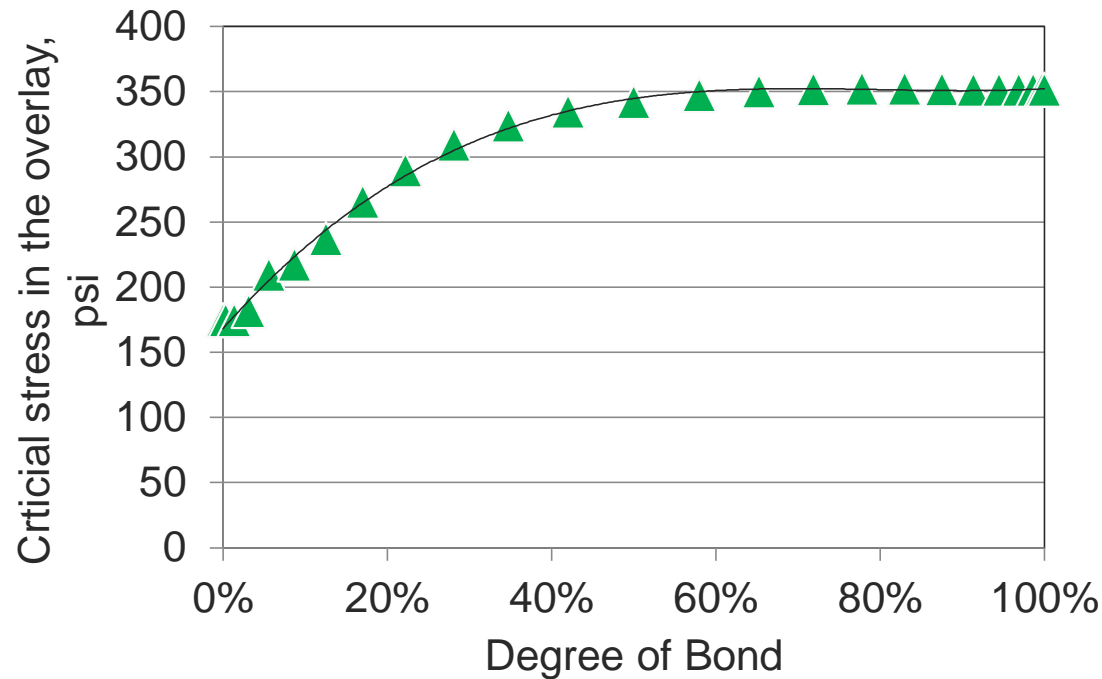
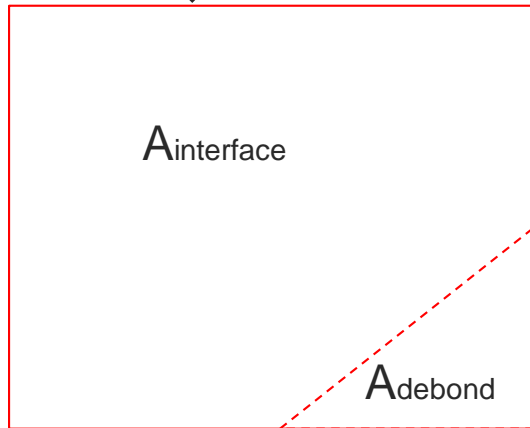
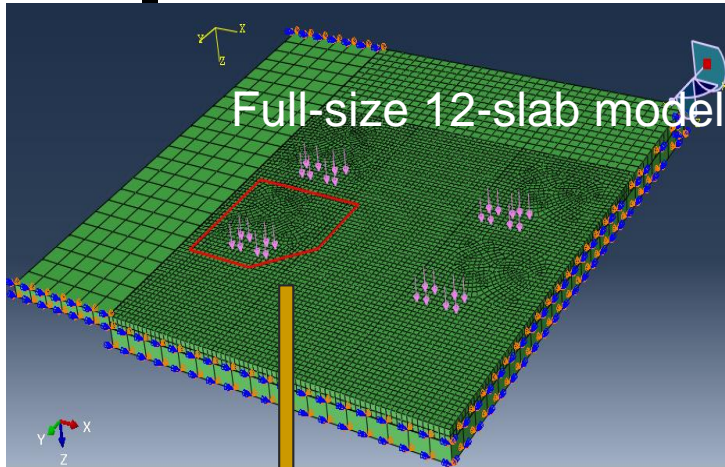
$$f = 1$$

**Unbonded**



$$f > 1$$

# Effect of partial bonding



$$DB\% = \frac{A_{debond}}{A_{interface}}$$

# [Considerations in current design]

	Degree of Bond (DB)
CDOT	Increase stress by 65%-59% <sup>1</sup>
NJDOT	Engineering judgment
PCA	Increase stress by 36% <sup>2</sup>
ICT	Same as PCA method

1. Based on Colorado data, 2. based on Missouri and Colorado Data

# Proposed debonding model

Paris' law:

$$A_{debond} = c \left( \frac{\Delta G}{G_c} \right)^m N$$

$\Delta G$  = Energy release rate, a function of applied load

$G_c$  = Critical energy release rate, a function of material testing

$c, m$  = Coefficients from slab testing

$N$  = Number of loads



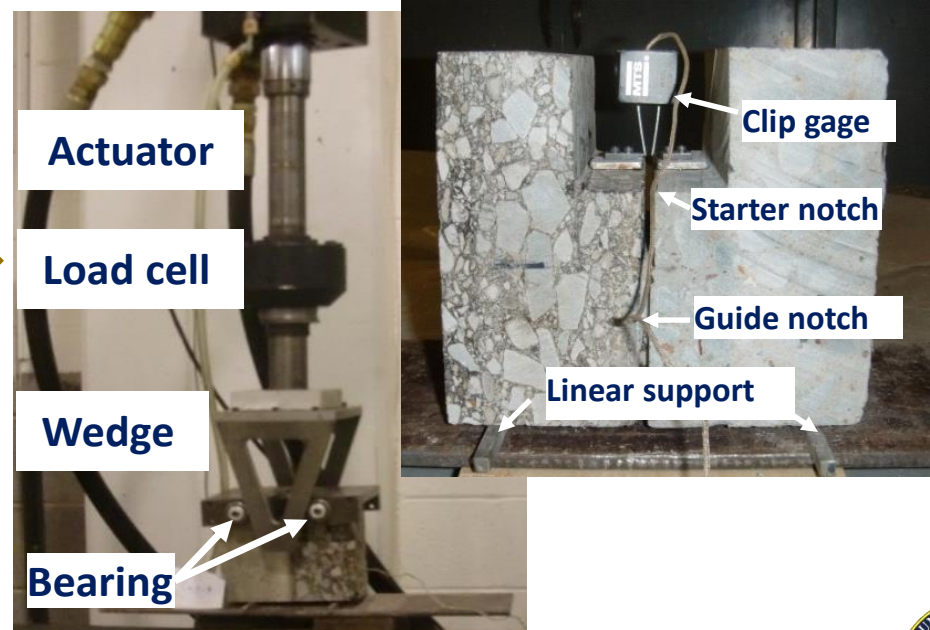
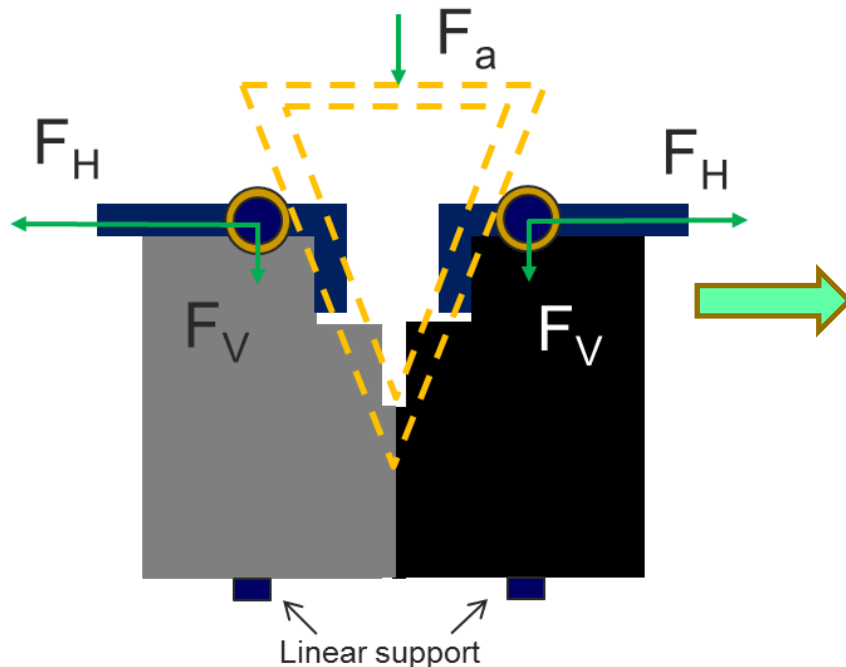
# Material Characterization

$$A_{debond} = c \left( \frac{\Delta G}{Gc} \right)^m N$$

$$\log Gc = B_0 + B_1 \log R + B_2 \log a_0$$

$R$ : Interface roughness

$a_0$ : Initial flaw size



# Calculation of debonding force

$$A_{debond} = c \left( \frac{\Delta G}{G_c} \right)^m N$$

$$\Delta G = f(L, h_{pcc}, h_{HMA}, E_{HMA}, P, A_{debond})$$

$L$ : Slab size

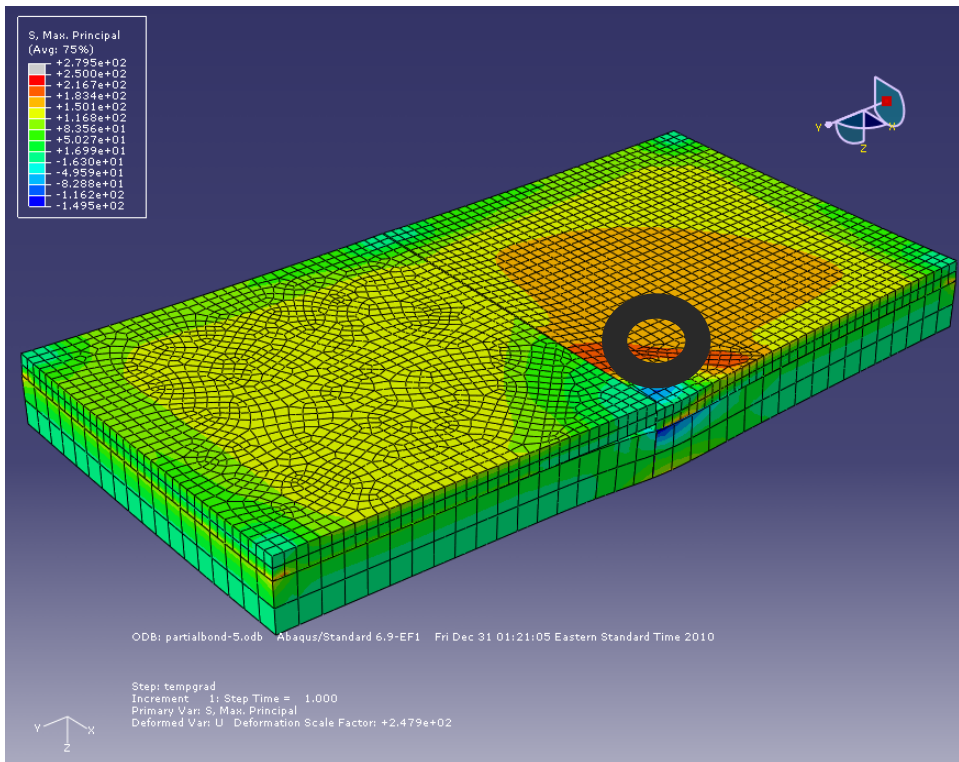
$h_{pcc}$ : Overlay thickness

$h_{HMA}$ : Asphalt thickness

$E_{HMA}$ : Asphalt stiffness

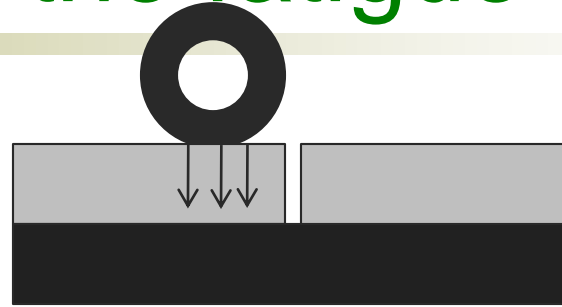
$P$ : load vector

$A_{debond}$ : current debonding size



# Calibration of the fatigue law

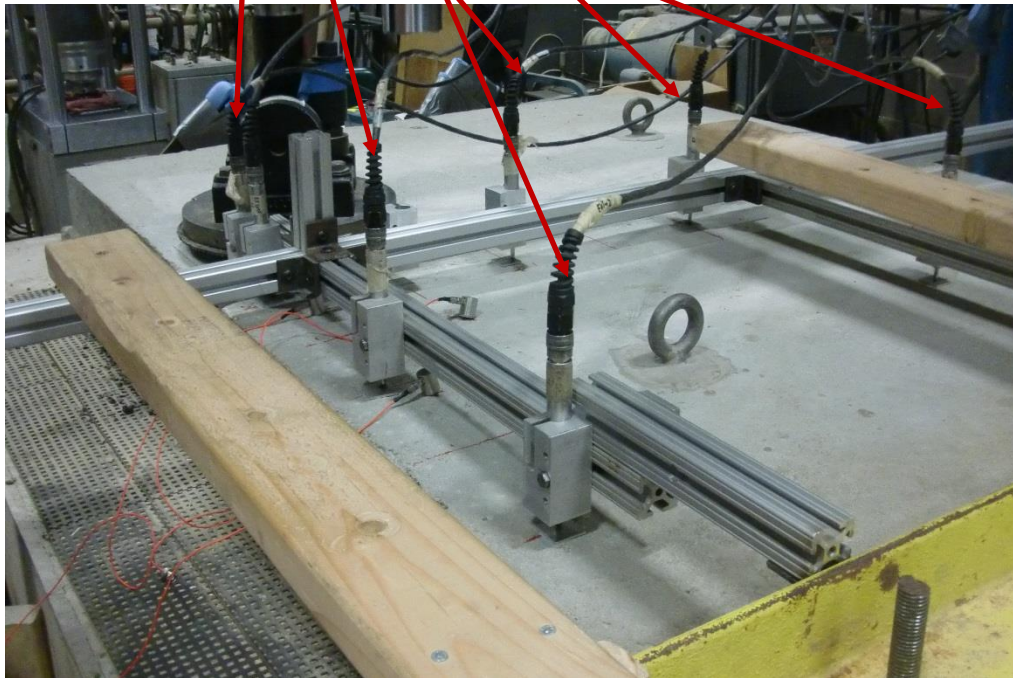
$$A_{debond} = c \left( \frac{\Delta G}{Gc} \right)^m N$$



# Methods to determine $A_{\text{debond}}$ @ N

$$A_{\text{debond}} = c \left( \frac{\Delta G}{Gc} \right)^m N$$

## 1. Deflection method



Deflections measured during testing of slabs



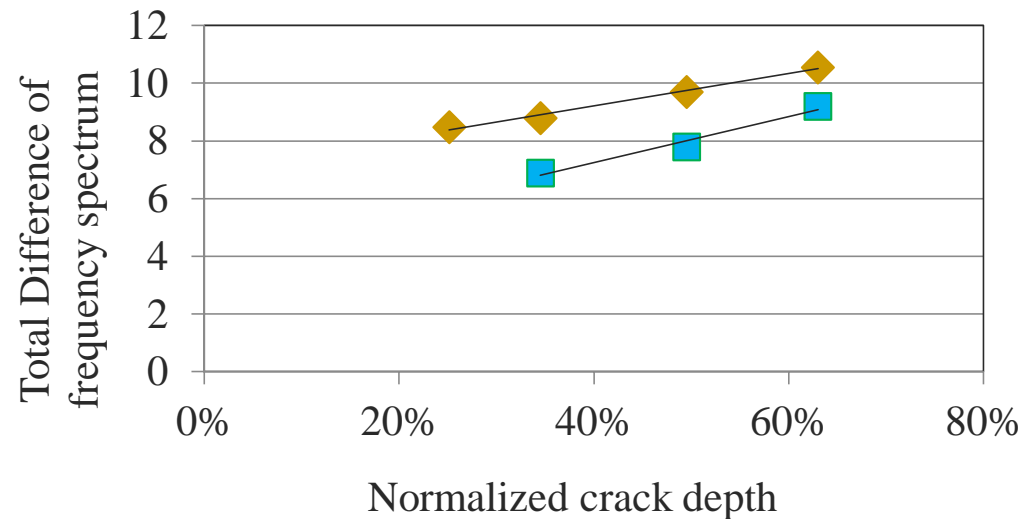
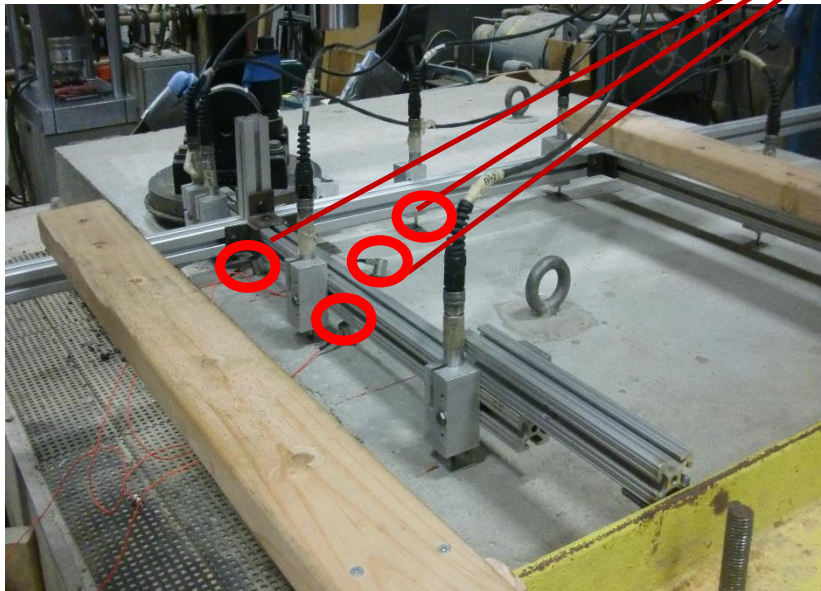
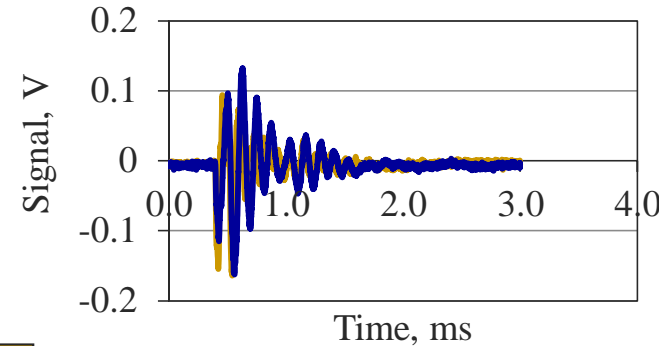
Deflections from FE models w/ various DB%



# Methods to determine $A_{\text{debond}}$ @ N

$$A_{\text{debond}} = c \left( \frac{\Delta G}{G_c} \right)^m N$$

## 2. Impact echo method



◆ PCC specimen    ■ PCC/HMA specimen